

数量総括表

総-1

[illegible]

数量総括表

総-2

[illegible]

数量総括表

総—3

[illegible]

数量総括表

総-4

[illegible]

数量総括表

総-5

エ 種 土工事集計						(1式) 当り
種 別	規 格	土 工 量			単位	備 考
		床掘	埋め戻し	発生土(盛土利用)		
セクション工						
A-1 バリア		1.47	0.60	0.80	m3	
B-1 スラッピー		0.11	0.00	0.11	m3	
C-1 スラッピー		0.06	0.00	0.06	m3	
E-3 レール		0.44	0.34	0.06	m3	
計		2.08	0.94	1.04	m3	
擁壁工						
重力式擁壁A	H=740	12.20	5.25	6.37	m3	
		$5.25 \times 2 = 10.50$	$2.35 \times 2 = 4.70$	$2.64 \times 2 = 5.28$		
重力式擁壁B	H=500~780	10.50	4.70	5.28	m3	
L型現場打擁壁①	H=500~1128	1.66	1.04	0.50	m3	
L型現場打擁壁②	H=500~1087	1.09	0.74	0.27	m3	
		$3.16 \times 2 = 6.32$	$1.81 \times 2 = 3.62$	$1.15 \times 2 = 2.30$		
L型現場打擁壁③	H=500~1140	6.32	3.62	2.30	m3	
計		31.77	15.35	14.71	m3	
付帯施設工						
	H=1.2m	$0.80 \times 29.10 / 10.00$	$0.72 \times 29.10 / 10.00$	$2.33 - 2.10 \div 0.90$		
フェンス新設	L=29.0m	2.33	2.10	0.00	m3	
片開き門扉新設	H=1.2m	0.68	0.58	0.04	m3	
両開き門扉新設	H=1.2m	0.86	0.72	0.06	m3	
パーゴラ新設		12.25	9.30	1.88	m3	
計		16.12	12.70	1.98	m3	
合計		49.97	28.99	17.73	m3	盛土量V=28.27m3

数量総括表

総-6

[illegible]

代価数量計算書

[illegible]

代価数量計算書

エ 種

D-2(プラットフォーム①)

(1ヶ所) 当り

平面図

重力式擁壁A

面積 A=14.614m2 (CAD計算)

16.050

12.531

擁壁延長(プラットフォーム下) $L=(16.050+12.531) \div 2=14.291$

プラットフォーム端部(重力式擁壁側)

転落防止柵

プラットフォーム

200

プラットフォーム

表層:コンクリート 24-12-20N

表面:滑らかに仕上げる

表面強化剤塗布

(スマートシリケート同等品)

D10タテヨコ#200

路盤:RC-40

盛土

1:0.6

90

▽±0

重力式擁壁A

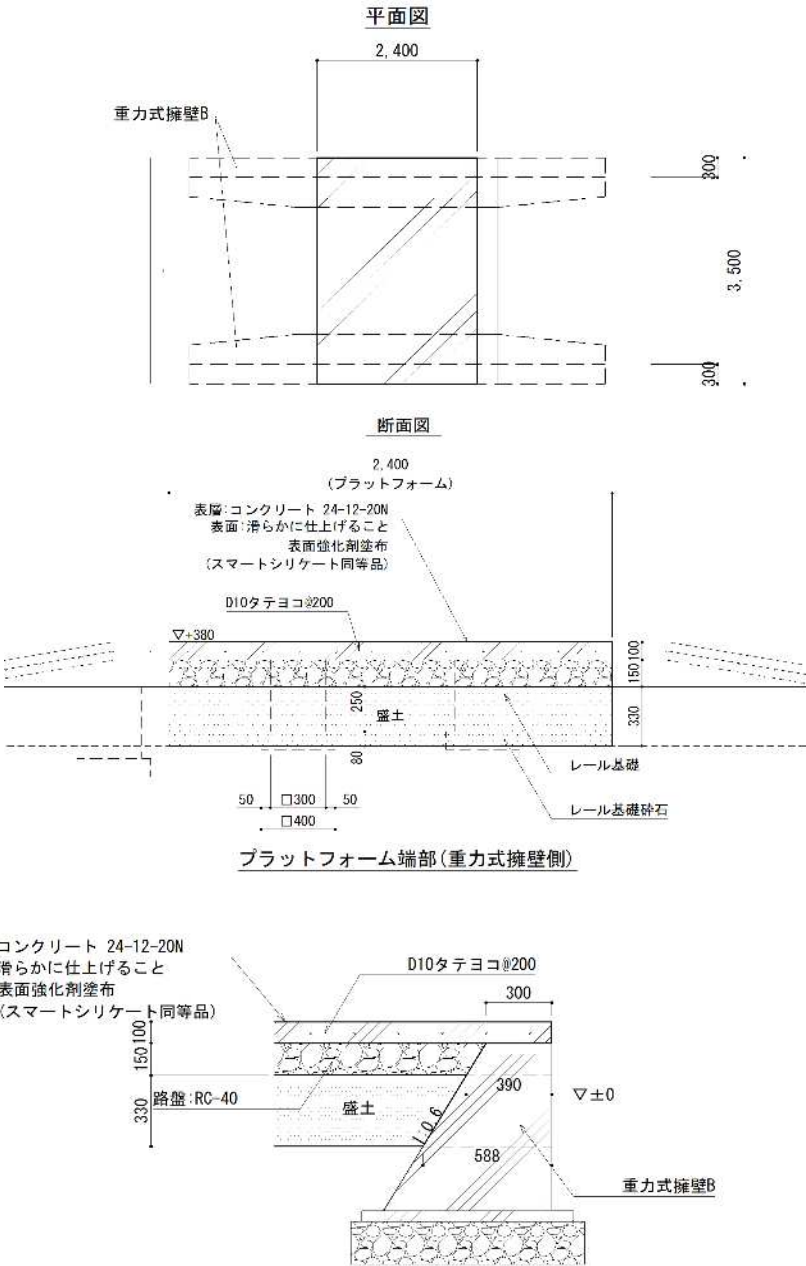
面積当たりの鉄筋重量

$\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000=5.600 \text{ (kg/m}^2\text{)}$

種 別	規 格	計 算 式	単位	数 量
		(面積はCAD計算による)		
盛土	発生土	$0.690 \times 14.291 - (0.090 + 0.504) \times 0.690 \div 2 \times 14.291$	m3	6.93
		(面積はCAD計算による)		
基面整正		$14.614 - (0.090 \times 14.291)$	m2	13.33
		(面積はCAD計算による)		
碎石路盤	RC-40 t=150	14.614	m2	14.61
		(面積はCAD計算による)		
鉄筋	D10	$14.614 \times 5.600 \text{ (kg/m}^2\text{)}$	Kg	81.84
	24-12-20N 鉄筋	(面積はCAD計算による)		
コンクリート	人力打設 特殊養生	14.614×0.10	m3	1.46
	コンクリート	(面積はCAD計算による)		
左官仕上げ	金ごて押え	14.614	m2	14.61
		(面積はCAD計算による)		
表面強化剤	スマートシリケート	14.614	m2	14.61

代 価 数 量 計 算 書

エ 種 E-1,2(プラットフォーム②) (1ヶ所) 当り



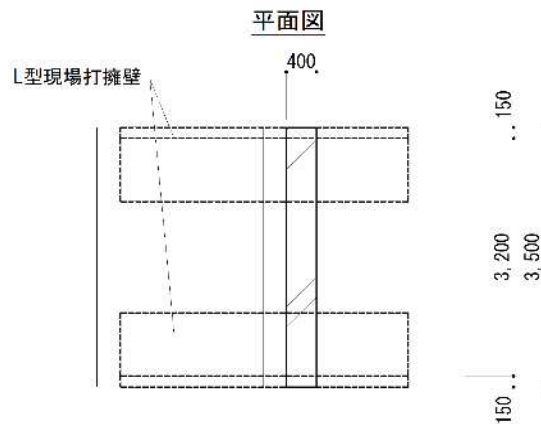
面積当たりの鉄筋重量 $\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$

種 別	規 格	計 算 式	単 位	数 量
盛土	発生土	$2.400 \times 3.500 \times 0.330 - (0.390 + 0.588) \times 0.330 \div 2 \times 2.400 \times 2 - (0.300 \times 0.300 \times 0.25 + 0.40 \times 0.40 \times 0.080) \times 2$	m3	1.93
基面整正		$2.400 \times 3.500 - (0.390 \times 2.400 \times 2)$	m2	6.53
碎石路盤	RC-40 t=150	$2.400 \times 3.500 - (0.300 \times 2)$	m2	7.80
鉄筋	D10	$2.400 \times 3.500 \times 5.600 \text{ (kg/m}^2\text{)}$	Kg	47.04
コンクリート	24-12-20N 鉄筋			
	人力打設 特殊養生	$2.400 \times 3.500 \times 0.1$	m3	0.84
	コンクリート			
左官仕上げ	金ごて押え	2.400×3.500	m2	8.40
表面強化剤	スマートシリケート	2.400×3.500	m2	8.40

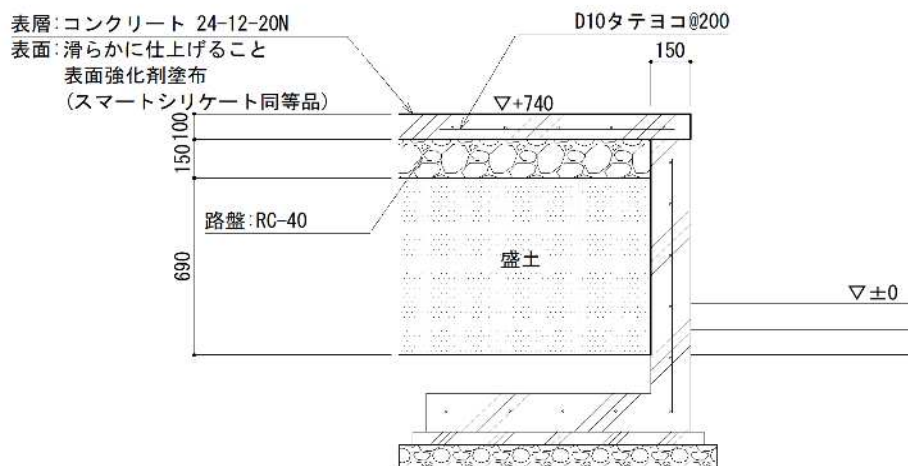
代 価 数 量 計 算 書

エ 種 F-1, 2(プラットフォーム①)

(1ヶ所) 当り



プラットフォーム端部(L型現場打擁壁側)



面積当たりの鉄筋重量 $\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$

種 別	規 格	計 算 式	単 位	数 量
盛土	発生土	$0.400 \times 3.200 \times 0.690$	m3	0.88
基面整正		0.400×3.200	m2	1.28
碎石路盤	RC-40 t=150	0.400×3.200	m2	1.28
鉄筋	D10	$0.400 \times 3.500 \times 5.600 \text{ (kg/m}^2\text{)}$	Kg	7.84
	24-12-20N 鉄筋			
コンクリート	人力打設 特殊養生	$0.400 \times 3.500 \times 0.1$	m3	0.14
	コンクリート			
左官仕上げ	金ごて押え	0.400×3.500	m2	1.40
表面強化剤	スマートシリケート	0.400×3.500	m2	1.40

代価数量計算書

エ 種 A-1 バリア

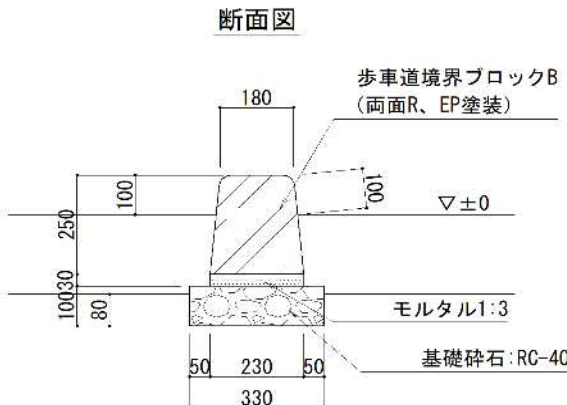
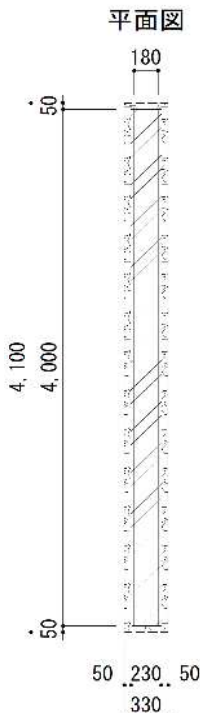
(1ヶ所) 当り

平面図

断面図

種 別	規 格	計 算 式	単位	数 量
床掘		$1.504 \times 4.600 \times 0.160 + 0.904 \times 4.000 \times 0.100$ (断面積はCAD計算による)	m ³	1.47
埋戻し		$(0.048 + 0.065) \times 4.000 + 1.504 \times 0.300 \times 0.160 \times 2$	m ³	0.60
残土処分		$1.47 - 0.60 \div 0.9$	m ³	0.80
基面整正		0.904×4.000	m ²	3.62
基礎碎石	RC-40 t=100	0.904×4.000	m ²	3.62
型枠	鉄筋	$0.441 \times 2 + ((0.360 + 0.740) + 1.390) \times 4.000$ $(12 \times (4.000 - 0.060 \times 2) + ((0.925 + 0.605 + 0.695 + 0.080)$ $\times (4.00 \div 0.200))) \times 0.560 \text{ (kg/m)}$	m ²	10.84
鉄筋	D10		kg	51.89
コンクリート	27-12-20N	0.441×4.00	m ³	1.76
表面仕上げ		$(0.740 + 0.100 + 0.848) \times 4.000$	m ²	6.75
表面強化剤	スマートシリケート	$(0.740 + 0.100 + 0.848) \times 4.000$	m ²	6.75

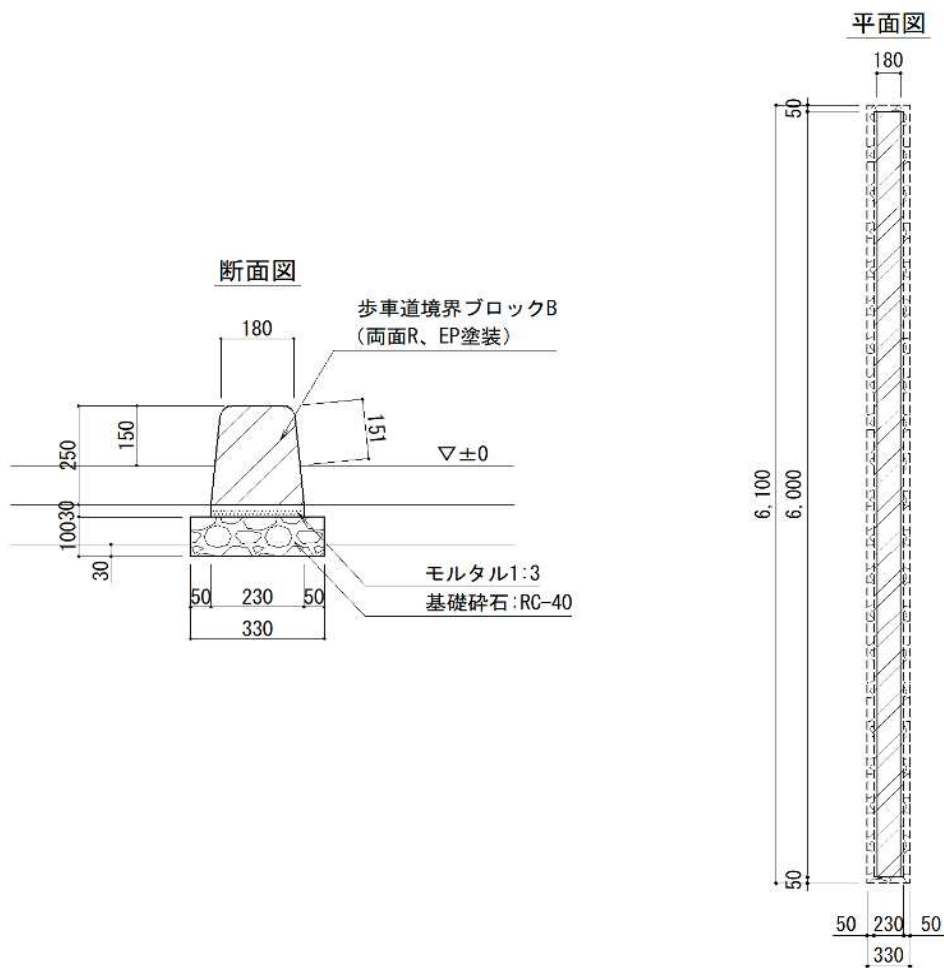
代 価 数 量 計 算 書

エ 種 B-1 スラッピー		(1ヶ所) 当り		
断面図  平面図 				
種 別	規 格	計 算 式	単位	数 量
床掘		$0.330 \times 4.100 \times 0.080$	m3	0.11
残土処分		$0.330 \times 4.100 \times 0.080$	m3	0.11
基面整正		0.330×4.100	m2	1.35
基礎碎石	RC-40 t=100	0.330×4.100	m2	1.35
モルタル	1:3	$0.230 \times 0.030 \times 4.000$	m2	0.03
歩車道境界ブロックB	両面R	$4.000 \div 0.610$	本	6.56
EP塗装	下地処理共	$(0.100 \times 2 + 0.180) \times 4.000$	m2	1.52

代 価 数 量 計 算 書

エ 種 C-1 スラッピー

(1ヶ所) 当り



種 別	規 格	計 算 式	単位	数 量
床掘		$0.330 \times 6.100 \times 0.030$	m3	0.06
残土処分		$0.330 \times 6.100 \times 0.030$	m3	0.06
基面整正		0.330×6.100	m2	2.01
基礎碎石	RC-40 t=100	0.330×6.100	m2	2.01
モルタル	1:3	$0.230 \times 0.030 \times 6.000$	m2	0.04
歩車道境界ブロックB	両面R	$6.000 \div 0.610$	本	9.84
EP塗装	下地処理共	$(0.151 \times 2 + 0.180) \times 6.000$	m2	2.89

代 価 数 量 計 算 書

-P. -

エ 種 D-1(バンクセクション部)

(1ヶ所) 当り

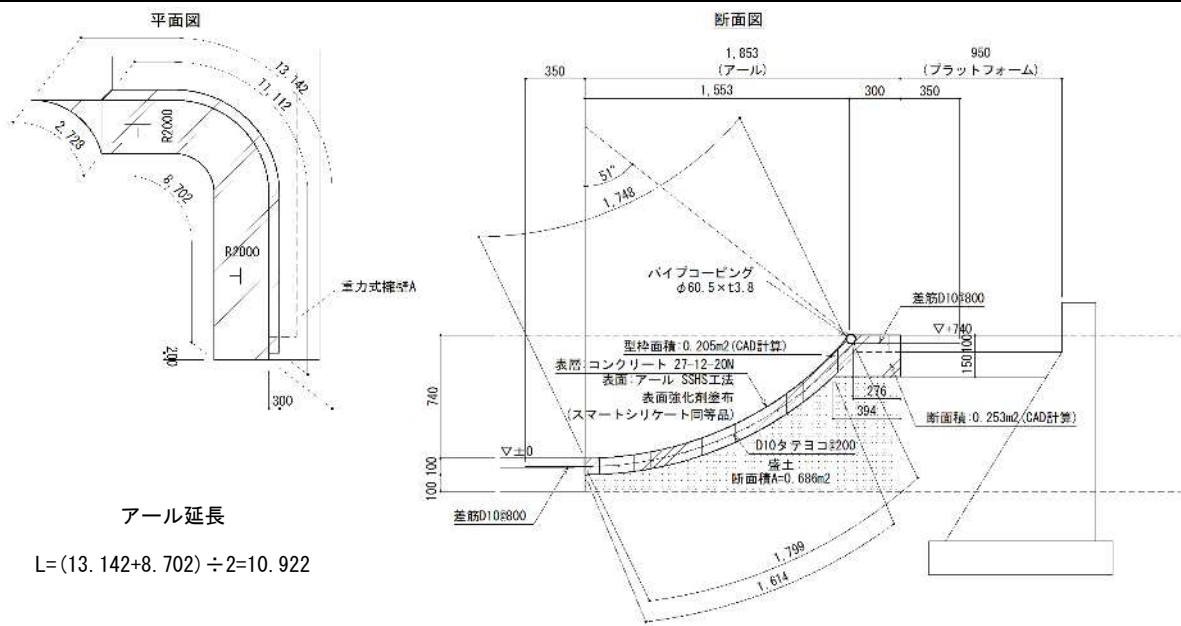
The figure contains three technical drawings of a bank section (D-1):

- 平面図 (Plan View):** Shows a rectangular area with dimensions 2,300 (total width), 2,000 (inner width), and 300 (offset). A 20° slope is indicated. A vertical dimension of 1,450 is shown, with a 1,300 segment and a 150 segment. A section line A-A' is marked. The area is labeled "面積 A=3.423m2 (CAD計算)".
- 断面図 (Cross-section View):** Shows a trapezoidal cross-section. The top width is 2,300 (バンク) and the bottom width is 2,000. The height is 350. A slope of 2.153 is shown. The cross-sectional area is "断面積: 0.244m2 (CAD計算)". The surface is "表層: コンクリート 27-12-20N" and "表面: バンク (滑らかに仕上げる)". A reinforcement layer is "表面強化剤塗布 (スマートシリケート同等品)" with an area of "型枠面積: 0.218m2 (CAD計算)". The bottom is "盛土①" with a cross-sectional area of "断面積A=0.896m2". A reinforcement bar "D10 タテヨコ #200" is shown. A platform "プラットフォーム" is at the top right with a height of 300. A vertical dimension of 740 is shown. A horizontal dimension of 950 is shown. A vertical dimension of 100 is shown. A horizontal dimension of 150 is shown. A vertical dimension of 100 is shown. A horizontal dimension of 100 is shown. A vertical dimension of 100 is shown. A horizontal dimension of 100 is shown. A vertical dimension of 100 is shown. A horizontal dimension of 100 is shown. A vertical dimension of 100 is shown. 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代 価 数 量 計 算 書

エ 種 D-2(アールセクション部)

(1ヶ所) 当り

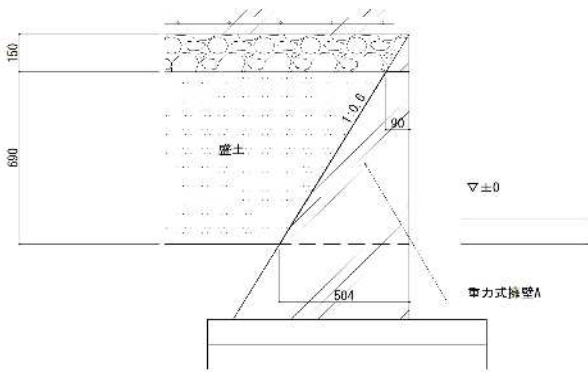


アール延長

$$L=(13.142+8.702) \div 2=10.922$$

フラットフォーム端部(重力式擁壁側)

フラットフォーム 200 フラットサーフェス



面積当たりの鉄筋重量 $\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$

種 別	規 格	計 算 式	単 位	数 量
盛土	発生土	$0.686 \times 10.922 - (0.090 + 0.504) \times 0.690 \div 2 \times 0.300$	m3	7.43
基面整正		$(1.614 + 0.394) \times 10.922$	m2	21.93
型枠	鉄筋	(断面積はCAD計算による) $0.205 + (0.100 + 0.150) \times 11.112 + 0.100 \times (2.728 + 8.702)$	m2	4.13
鉄筋	D10	$(1.748 + 0.300) \times 10.922 \times 5.600 \text{ (kg/m}^2\text{)}$	kg	125.26
差筋	D10	$(0.350 \times 11.112 \div 0.800 +$ $0.350 \times (2.728 + 8.702) \div 0.800) \times 0.560 \text{ (kg/m)}$	kg	5.52
コンクリート	27-12-20N t=100	(断面積はCAD計算による) 0.253×10.922	m3	2.76
表面仕上げ	SSHS工法	$(1.748 + 0.300) \times 10.922 - 0.200 \times 0.300$	m2	22.31
表面強化剤	スマートシリケート	$(1.748 + 0.300) \times 10.922 - 0.200 \times 0.300$	m2	22.31

代価数量計算書

[illegible]

代価数量計算書

エ 種 E-2(バンクセクション部)

(1ヶ所) 当り

平面図

断面図

重力式擁壁B側面図

A-A' 断面図
プラットフォーム プラットサーフェス

B-B' 断面図
プラットフォーム プラットサーフェス

面積当たりの鉄筋重量 $\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$

種 別	規 格	計 算 式	単位	数 量
		(断面積はCAD計算による)		
盛土	発生土	$0.670 \times 3.500 - ((0.300 + 0.588) \times 0.480 \div 2 \times 1.621 + (0.300 + 0.504) \times 0.340 \div 2 \times 0.291) \times 2$	m ³	1.57
基面整正		$(1.360 + 1.160) \times 3.500$	m ²	8.82
		(断面積はCAD計算による)		
型枠	鉄筋	$(0.253 \times 2 + (0.100 + 0.150) \times 3.500 + 0.100 \times 3.500)$	m ²	1.73
鉄筋	D10	$(2.233 + 0.300) \times 3.5 \times 5.600 \text{ (kg/m}^2\text{)}$	kg	49.65
差筋	D10	$2 \times 0.350 \times (3.500 \div 0.800) \times 0.560 \text{ (kg/m)}$	kg	1.72
	27-12-20N 鉄筋			
コンクリート	人力打設 特殊養生	$(2.233 + 0.300) \times 3.500 \times 0.1$	m ²	0.89
	コンクリート			
左官仕上げ	金ごて押え	$(2.233 + 0.300) \times 3.500$	m ²	8.87
表面強化剤	スマートシリケート	$(2.233 + 0.300) \times 3.500$	m ²	8.87

代価数量計算書

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代価数量計算書

エ

種

F-2(アールセクション部)

(1ヶ所) 当り

面積当たりの鉄筋重量				
$\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$				
種 別	規 格	計 算 式	単位	数 量
盛土	発生土	0.686×3.500	m3	2.40
基面整正		$(1.614 + 0.394) \times 3.500$ (断面積はCAD計算による)	m2	7.03
型枠	鉄筋	$0.205 \times 2 + (0.100 + 0.150) \times 3.500 + 0.100 \times 3.500$	m2	1.64
鉄筋	D10	$(1.748 + 0.300) \times 3.500 \times 5.600 \text{ (kg/m}^2\text{)}$	kg	40.14
差筋	D10	$2 \times 0.350 \times (3.500 \div 0.800) \times 0.560 \text{ (kg/m)}$	kg	1.72
	27-12-20N	(断面積はCAD計算による)		
コンクリート	t=100	0.253×3.500	m3	0.89
表面仕上げ	SSHS工法	$(1.748 + 0.300) \times 3.500$	m2	7.17
表面強化剤	スマートシリケート	$(1.748 + 0.300) \times 3.500$	m2	7.17

代価数量計算書

工 種

D-2(重力式擁壁A)

(1ヶ所) 当り

平面図

断面図

擁壁延長

$L = (16.650 + 13.114) \div 2 = 14.882$

捨てコンクリート・碎石延長

$L = (16.850 + 13.314) \div 2 = 15.082$

床掘り延長

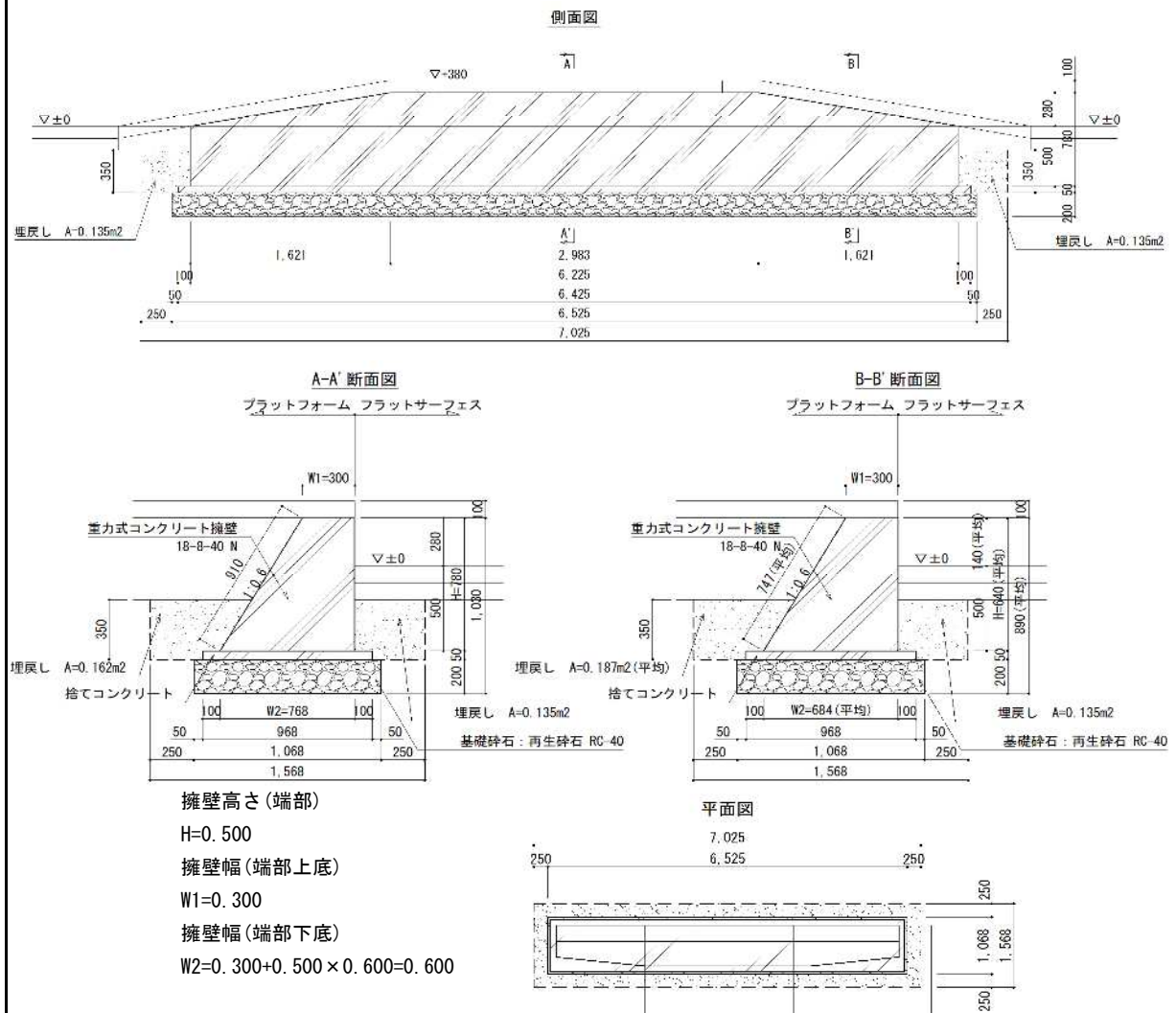
$L = (17.450 + 13.914) \div 2 = 15.682$

種 別	規 格	計 算 式	単位	数 量
床掘		$1.684 \times 0.400 \times 15.682 + 1.084 \times 0.100 \times 15.082$	m3	12.20
埋戻し		$(0.177 + 0.150) \times 14.882 + (1.684 \times 0.400 \times 0.300 - 1.084 \times 0.100 \times 0.100) \times 2$	m3	5.25
残土処分		$12.20 - 5.25 \div 0.9$	m3	6.37
基面整正		0.884×15.082	m2	13.33
基礎碎石	RC-40 t=100	0.884×15.082	m2	13.33
均しコンクリート				
型枠	均し	$(15.082 \times 0.100 + 1.084 \times 0.100) \times 2$	m2	3.23
均しコンクリート	18-8-25BB	$1.084 \times 0.100 \times 15.082$	m3	1.63
型枠	無筋	$0.678 \times 2 + (1.440 + 1.329 + 0.300) \times 14.882$	m2	47.03
コンクリート	18-8-40N	0.678×14.882	m3	10.09
等辺山形鋼	SS400 6*50*50 丸面取り	単位体積重量: 4.430kg/m $(16.650 + 16.050) \times 4.430$	kg	144.86

代価数量計算書

工 種 E-1, 2(重力式擁壁B)

(1ヶ所) 当り



種 別	規 格	計 算 式	単位	数 量
床掘		$1.568 \times 7.025 \times 0.350 + 1.068 \times 6.525 \times 0.200$	m ³	5.25
		(断面積はCAD計算による)		
埋戻し		$(0.162 + 0.135) \times 2.983 + (0.187 + 0.135) \times 1.621 \times 2 + 0.135 \times 1.568 \times 2$	m ³	2.35
残土処分		$5.25 - 2.35 \div 0.900$	m ³	2.64
基面整正		1.068×6.525	m ²	6.97
基礎碎石	RC-40 t=100	1.068×6.525	m ²	6.97
均しコンクリート				
型枠	均し	$(6.425 \times 0.050 + 0.968 \times 0.050) \times 2$	m ²	0.74
均しコンクリート	18-8-25BB	$0.968 \times 6.425 \times 0.050$	m ³	0.31
型枠	無筋	$(0.780 + 0.910) \times 2.983 + (0.640 + 0.747) \times 1.621 \times 2 + (0.300 + 0.600) \times 0.500 \div 2 \times 2$	m ²	9.99
コンクリート	18-8-40N	$(0.300 + 0.684) \times 0.640 \div 2 \times 1.621 \times 2$	m ³	2.26

代価数量計算書

-P. -

工 種		D-1 (L型現場打擁壁①)			(1ヶ所) 当り
平面図		側面図			
断面図		断面図			
面積当たりの鉄筋重量		$\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$			
種 別	規 格	計 算 式	単位	数 量	
床掘		$1.700 \times 2.410 \times 0.350 + 1.200 \times 1.910 \times 0.100$ (断面積はCAD計算による)	m3	1.66	
埋戻し		$0.367 \times 1.710 + 0.120 \times 1.700 \times 2$	m3	1.04	
残土処分		$1.66 - 1.04 \div 0.9$	m3	0.50	
基面整正		1.200×1.910	m2	2.29	
基礎砕石	RC-40 t=100	1.200×1.910	m2	2.29	
均しコンクリート					
型枠	均し	$(1.81 + 1.100) \times 0.050 \times 2$	m2	0.29	
均しコンクリート	18-8-25BB	$1.810 \times 1.100 \times 0.050$	m3	0.10	
型枠	鉄筋	$(0.150 + 0.664) \times 1.710 \times 2 + 0.150 \times (0.500 + 1.128 + 0.850 \times 2)$	m2	3.28	
鉄筋	D10	$(1.392 + 0.850 \times 1.710) \times 5.600 \text{ (kg} \div \text{m}^2\text{)}$ (断面積はCAD計算による)	kg	15.93	
コンクリート	24-8-20N	$1.392 \times 0.150 + 0.850 \times 1.710 \times 0.150$	m3	0.43	

代価数量計算書

工 種

D-2 (L型現場打擁壁②)

(1ヶ所) 当り

平面図

側面図

断面図

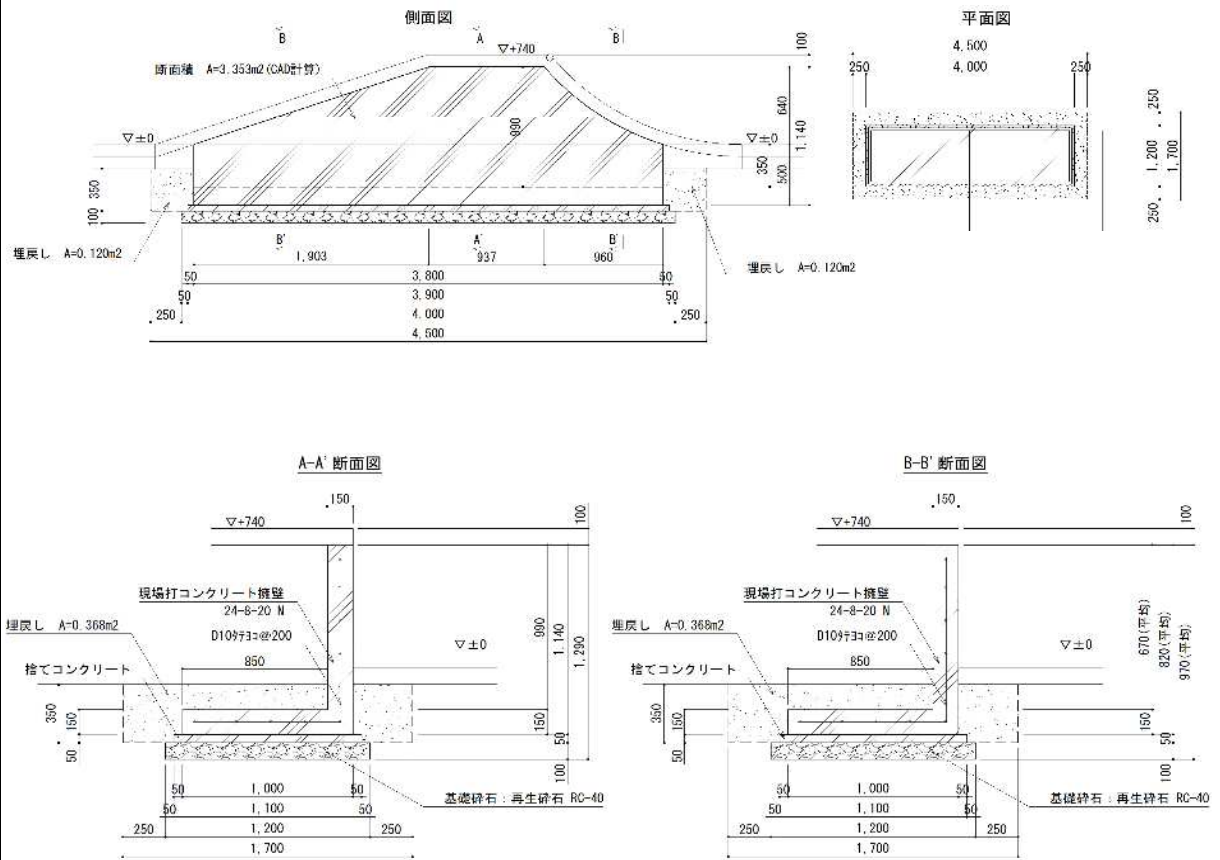
面積当たりの鉄筋重量 $\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$

種 別	規 格	計 算 式	単 位	数 量
床掘		$1.700 \times 1.613 \times 0.350 + 1.200 \times 1.113 \times 0.100$ (断面積はCAD計算による)	m3	1.09
埋戻し		$0.367 \times 0.913 + 0.120 \times 1.700 \times 2$	m3	0.74
残土処分		$1.09 - 0.74 \div 0.9$	m3	0.27
基面整正		1.200×1.113	m2	1.34
基礎砕石	RC-40 t=100	1.200×1.113	m2	1.34
均しコンクリート				
型枠	均し	$(1.013 \times 0.050 + 1.100 \times 0.050) \times 2$	m2	0.21
均しコンクリート	18-8-25BB	$1.013 \times 1.100 \times 0.050$	m3	0.06
型枠	鉄筋	$(0.150 + 0.644) \times 2 \times 0.913 + 0.150 \times (0.500 + 1.087 + 0.850 \times 2)$	m2	1.94
鉄筋	D10	$(0.672 + 0.850 \times 0.913) \times 5.600 \text{ (kg/m}^2\text{)}$ (断面積はCAD計算による)	kg	8.11
コンクリート	24-8-20N	$0.672 \times 0.150 + 0.850 \times 0.913 \times 0.150$	m3	0.22

代価数量計算書

工 種 F-1, 2 (L型現場打擁壁③)

(1ヶ所) 当り



面積当たりの鉄筋重量 $\sqrt{10} \times (\sqrt{10} \div 0.200) \times 2 \times 0.560 \div 10.000 = 5.600 \text{ (kg/m}^2\text{)}$

種 別	規 格	計 算 式	単位	数 量
床掘		$1.700 \times 4.500 \times 0.350 + 1.200 \times 4.000 \times 0.100$ (断面積はCAD計算による)	m ³	3.16
埋戻し		$0.368 \times 3.800 + 0.120 \times 1.700 \times 2$	m ³	1.81
残土処分		$3.16 - 1.81 \div 0.9$	m ³	1.15
基面整正		1.200×4.000	m ²	4.80
基礎砕石	RC-40 t=100	1.200×4.000	m ²	4.80
均しコンクリート				
型枠	均し	$(3.900 \times 0.050 + 1.100 \times 0.050) \times 2$	m ²	0.50
捨てコンクリート	18-8-25BB	$1.100 \times 3.900 \times 0.050$	m ³	0.21
型枠	鉄筋	$(0.150 \times 2 \times 3.800 + 0.990 \times 2 \times 0.937 + 0.670 \times 2 \times (1.903 + 0.960) + 0.150 \times (0.500 + 0.850) \times 2$	m ²	7.24
鉄筋	D10	$((3.353 + 0.850 \times 3.800)) \times 5.600 \text{ (kg/m}^2\text{)}$	kg	36.86
	鉄筋	(断面積はCAD計算による)		
コンクリート	24-8-20N	$3.353 \times 0.150 + 0.850 \times 0.150 \times 3.800$	m ³	0.99

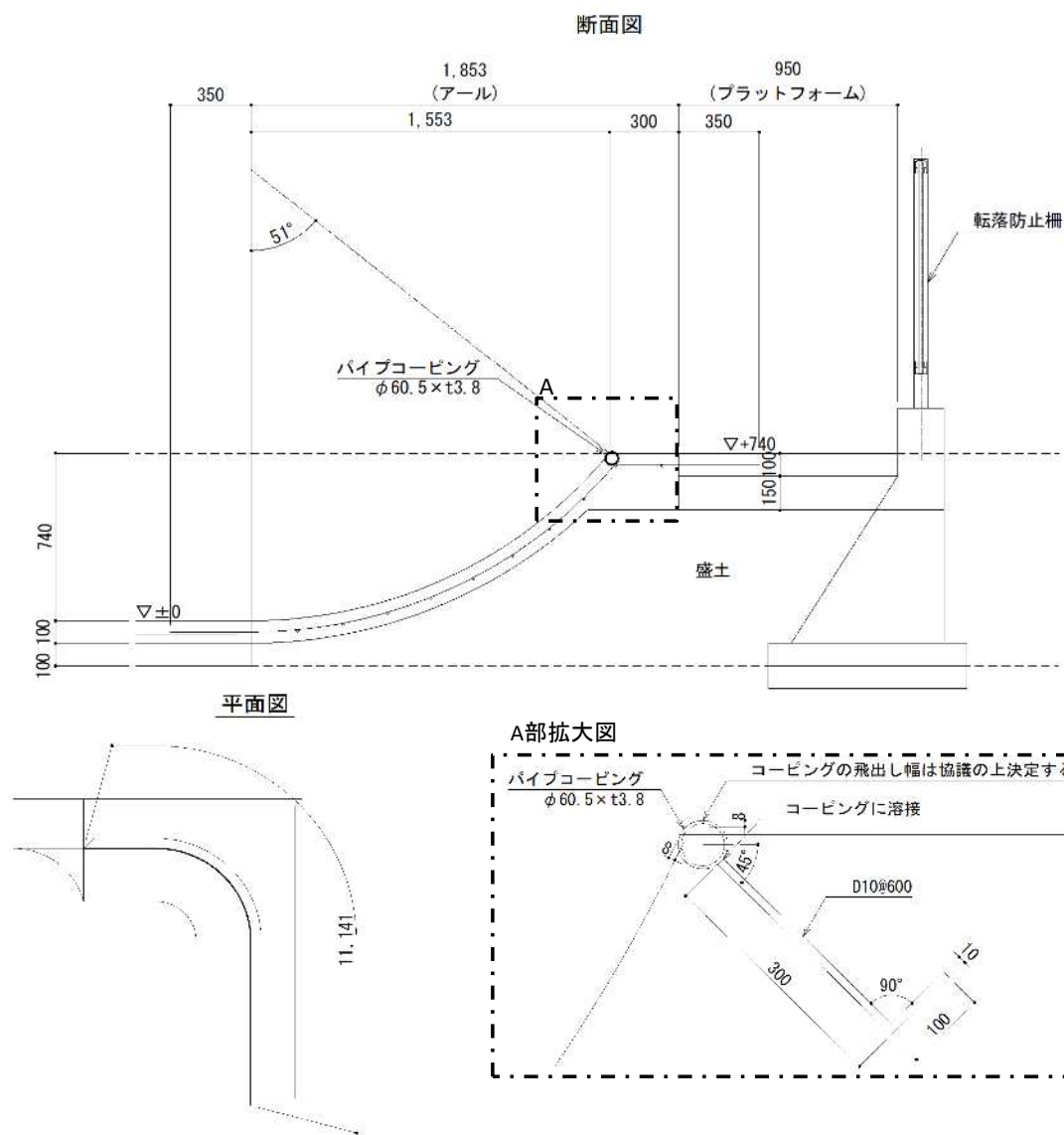
代価数量計算書

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代価数量計算書

エ 種 D-2 (コーピング部)

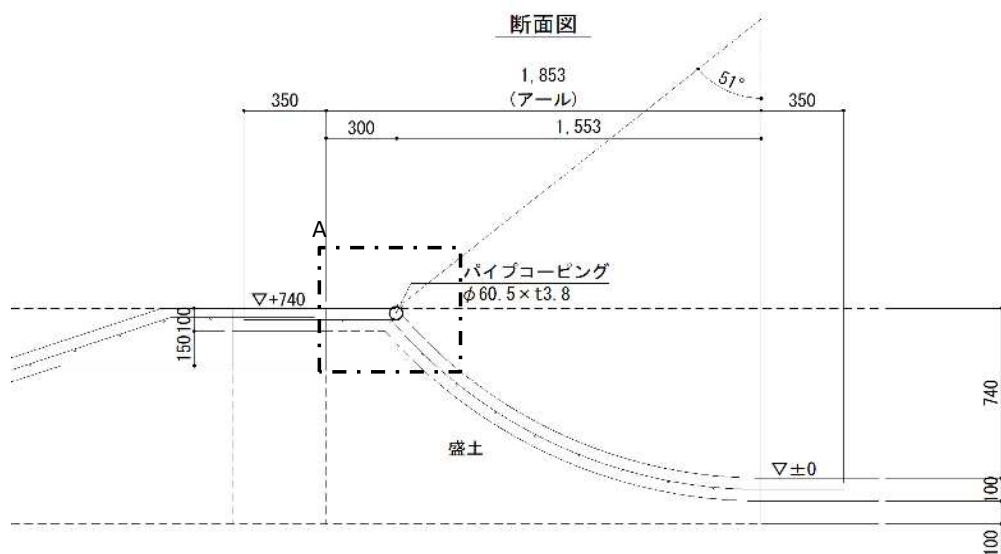
(1ヶ所) 当り

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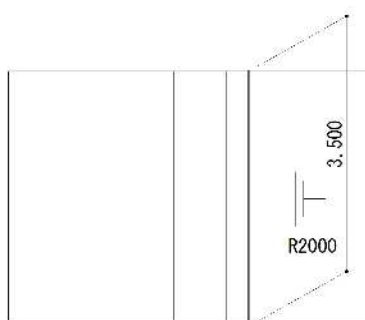
代価数量計算書

エ 種 F-2 (コーピング部)

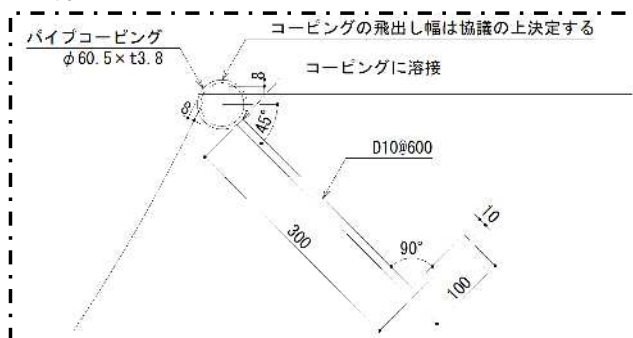
(1ヶ所) 当り



平面图



A部拡大図

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代価数量計算書

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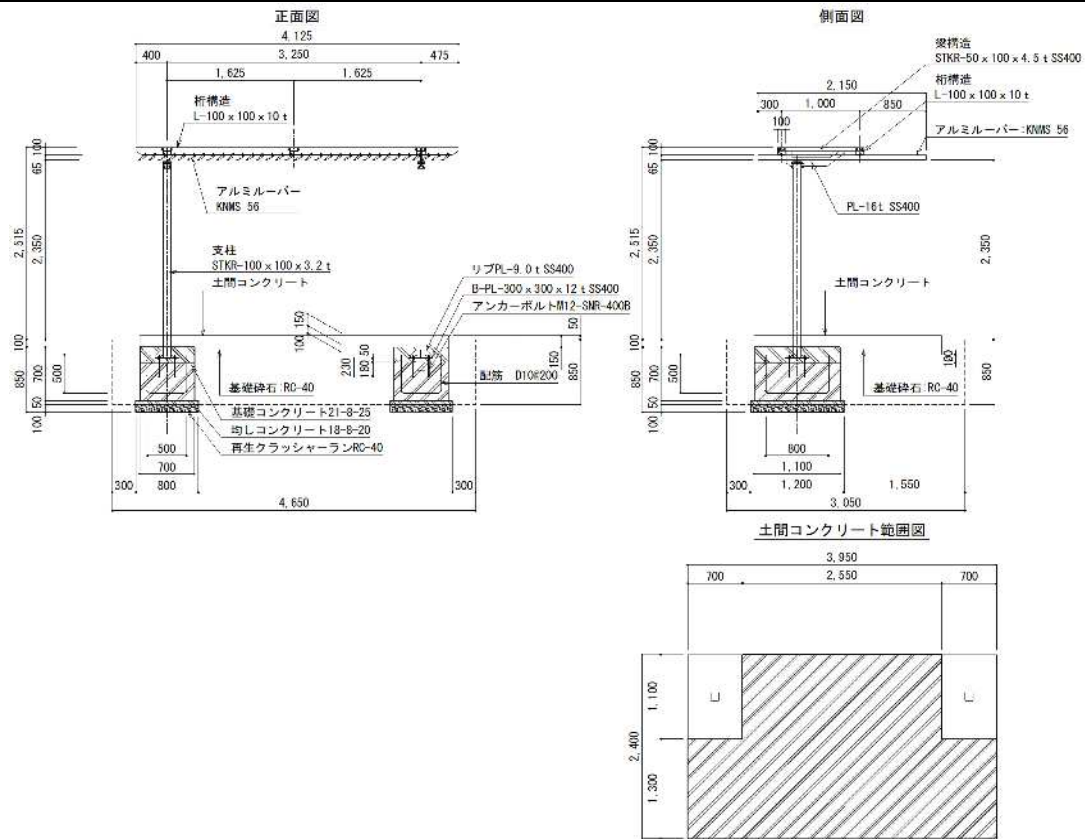
代価数量計算書

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代価数量計算書

エ 種 パーゴラ (1/2)

(1基) 当り



種 別	規 格	計 算 式	単 位	数 量
床掘		$4.650 \times 3.050 \times 0.850 + 0.800 \times 1.200 \times 0.100 \times 2$	m3	12.25
埋戻し		$12.25 - ((3.950 \times 2.400 - 0.700 \times 1.100 \times 2) \times (0.100 + 0.100) + 0.700 \times 1.100 \times 0.700 \times 2 + 0.800 \times 1.200 \times (0.050 + 0.100) \times 2)$	m3	9.30
残土処分		$12.25 - 9.33 \div 0.900$	m3	1.88
基面整正		$0.800 \times 1.200 \times 2$	m2	1.92
基礎砕石	RC-40 t=100	$0.800 \times 1.200 \times 2$	m2	1.92
型枠	均し	$(0.800 + 1.200) \times 0.050 \times 2 \times 2$	m2	0.40
コンクリート	均しコンクリート 18-8-20	$0.800 \times 1.200 \times 0.050 \times 2$	m3	0.10
型枠	鉄筋	$(0.700 + 1.100) \times 0.700 \times 2 \times 2$	m2	5.04
コンクリート	基礎コンクリート 21-8-25N	$0.700 \times 1.100 \times 0.700 \times 2$	m3	1.08
鉄筋	D10	$((0.500 \times 3) + (0.500 \times 2 + 0.800)) \times 4 \times 2 \times 0.56$	kg	14.78
基面整正		$3.950 \times 2.400 - 0.700 \times 1.100 \times 2$	m2	7.94
基礎砕石	RC-40 t=100	$3.950 \times 2.400 - 0.700 \times 1.100 \times 2$	m2	7.94
型枠	小型	$(3.950 + 2.400 + (0.700 + 1.100) \times 2) \times 0.150 \times 2$	m2	2.99
コンクリート	土間コンクリート 18-8-25	$(3.950 \times 2.400 - 0.700 \times 1.100 \times 2) \times 0.150$	m3	1.19

代価数量計算書

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代価数量計算書

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代価数量計算書

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